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QUASI-STATIC AND HIGH-DYNAMIC COMPRESSIVE STRENGTH TESTING OFYOUNG AND MATURE CEMENT PASTE

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Motivation

The ultimate load carrying capacity of cementitious materials increases with increasing loading rate. Notably, the micro-structures of cementitious materials are glued together by the same binder, which is cement paste, representing a mix of cement and water at specific water-to-cement mass ratios w/c. Improved insight into the loading rate-dependent compressive strength behavior of cement paste will allow for a better understanding of cementitious materials, in general.

We here report on two experimental campaigns involving destructive uniaxial compressive strength tests on cylindrical cement paste samples [1]. The tests were designed such as to shed light on the material effects responsible (i) for the moderate strength increase in the regime of quasi-static loading rates and (ii) for the significant strength increase in the regime of high-dynamic loading rates. Thereby, we consider that the water-saturation level is known to influence the mechanical performance of cementitious materials. This is the motivation to investigate two markedly different materials [1].

Young, water-saturated cement paste

In order to study a practically water-saturated cement paste, we produced samples with an initial water-to-cement mass ratio w/c = 0.43. During the first 24 hours after production, cylindrical specimens were stored in sealed formworks, in a climate chamber conditioned to 25 degrees. One day after production, the specimens were demolded and their end faces were processed in order to achieve co-planarity. After that, the specimens were stored in limesaturated solution, conditioned to 25 degrees centigrade. Tests were carried out 48 hours after production.

Materials and Experimental Methods

Mature, oven-dried cement paste

In order to study a practically dry cement paste, we produced samples with an initial water-tocement mass ratio w/c = 0.60. For a few days, they were stored in sealed formworks. After that, they air-cured for six months. Right before testing, the specimens were oven-dried for 20 hours, at 75°C. This removed most of the free water from the specimens. Tests were carried out once the specimens had cooled down to room temperature.

Quasi-static testing

Quasi-static uniaxial compressive strength tests were carried out on a conventional universal electro-mechanical testing machine of type Walter and Bai LFM 150, both types of cement paste, on cylindrical specimens with a diameter of 30 mm and a height of 60 mm. The experiments were performed under force control, with stress rates ranging from 1.5x10⁻² MPa/s to 8.2x10⁺¹ MPa/s, covering four orders of magnitude.

High-dynamic testing

High-dynamic uniaxial compressive strength tests were carried out on a split-Hopkinson pressure bar. The mature and oven-dried cement paste, on cylindrical specimens with a diameter of 10.1 mm and a height of 6.6 mm. Effective high-dynamic strain rates ranged from $2 \times 10^{+2} \text{ s}^{-1}$ to 5x10⁺³ s⁻¹.

Results, Modeling, and Conclusions

Quasi-static strength of

young, water-saturated cement paste



Experimental results:

Strength increases moderately with increasing quasi-static stress rate Crack propagation in loading direction Modeling: viscoelasto-brittle approach [1] Quasi-static and high-dynamic strength of mature, oven-dried cement paste

strain rate: $\dot{\varepsilon}$ [1/s] $10^{-5} 10^{-4} 10^{-3} 10^{-2} 10^{-1} 10^{0} 10^{1} 10^{2} 10^{3} 10^{4}$ rease [-] \Box test results

Experimental results:

No quasi-static strengthening

crack to split the sample

Significant high-dynamic strengthening

Crack propagation in loading direction

Time to peak load = time to nucleation of

Critical strain criterion for crack nucleation

first crack + time required for the first

• Crack propagation at Rayleigh wave speed

Interpretation: Measured strength data

Modeling: elasto-brittle approach [1]

 $f_{c,ini} \dots$ initial strength of cement paste, prior to load-induced damage associated with creep deformation

Note: all figures after [2]

- Non-linear creep model accounting for damage associated with creep
- Ultimate strain criterion for strength

Interpretation: Strength decreases with decreasing loading rate, because the test duration is increased, and this provides creep associated damage mechanisms with more time to reduce the initially available strength

High-dynamic strength model validation at mortar scale



... at concrete scale



 $s_{prop} = 1.0 \ h \dots$ first crack nucleates at specimen surface d = maximum aggregate diameter h = specimen size in direction of crack propagation E = Young's modulus $f_{c,qs} =$ quasi-static strength μ = shear modulus ρ = mass density

Mortar Similar to cement paste: peak load right before first crack splits the sample. **Concrete** Maximum aggregate size matters. Peak load is reached, once the first crack



experimental data from [1]

 $s_{prop} = 0.5 h \dots$ first crack nucleates at specimen center $s_{prop} = 1.0 h \dots$ first crack nucleates at specimen surface \vec{h} = specimen size in direction of crack propagation E = Young's modulus $f_{c,qs} =$ quasi-static strength μ = shear modulus ρ = mass density

Conclusions

scatter stems from uncertainty regarding the position where the first crack nucleates

Quasi-static tests provide enough time for creep to be significant. Crack propagation (i.e. "failure") of the specimen is a quasi-instantaneous effect. Creep activity increases with water content. Creep is associated with damage. Strength decreases with decreasing loading rate, because the test duration is increased, and this provides creep associated damage mechanisms with more time to reduce the initially available strength. High-dynamic tests do not provide enough time for creep to be significant. Crack propagation at Rayleigh wave speed is not a quasi-instantaneous effect. During crack growth in loading direction, material columns form between the cracks, and their loading can be further increased, until the first crack splits the specimen (cement paste and mortar). The concrete peak load is reached, once the first aggregate breaks out of the concrete microstructure. We conclude that high dynamic strengthening is a structural effect rather than a material property.

References:

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[3] Gary, G. & P. Bailly (1998). Behaviour of quasi-brittle material at high strain rate. Experiment and modelling. European Journal of







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